



Hydraulic structures. Dams and reservoirs

Dam outlet works and Energy dissipation - 1

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Strengthening of master curricula in water resources
management for the Western Balkans HEIs and stakeholders

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HYDROENGINEERING STRUCTURES-4



Dam Outlet Works and Energy Dissipation

1. Introduction
2. Design flood
3. Spilways
4. Bottom outlets
5. Energy dissipation



Introduction

Dams outlet works

in general

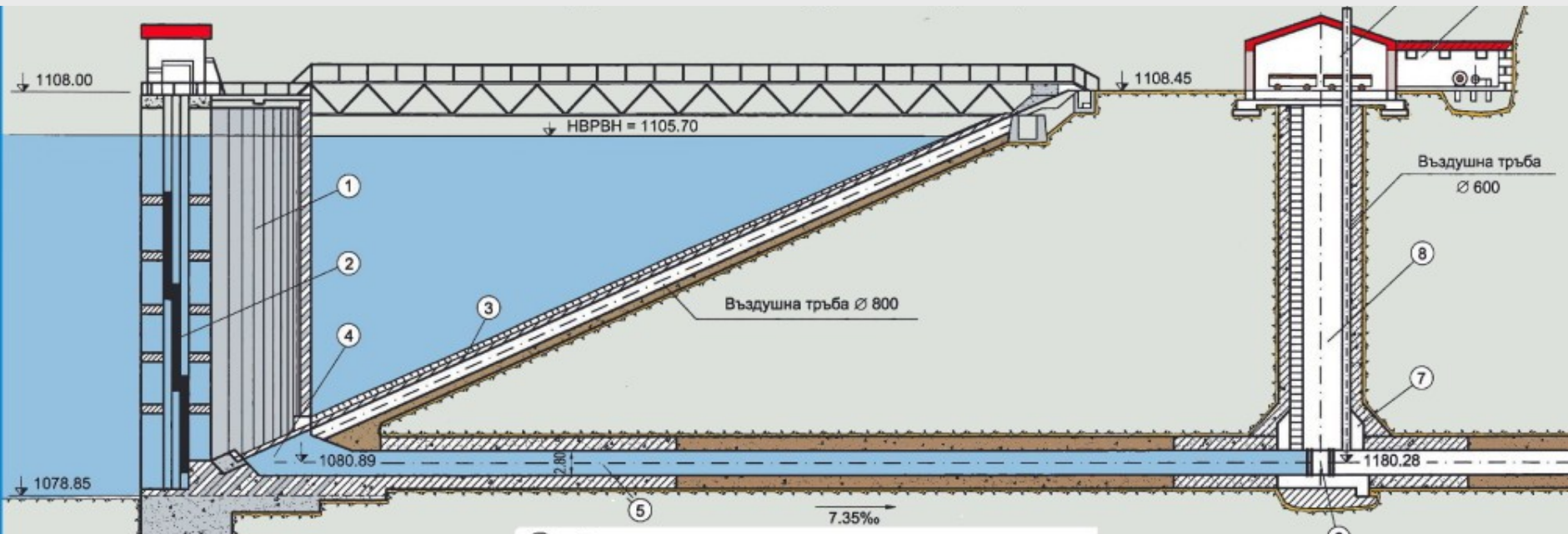
Spillways



Bottom outlet



Other outlet works

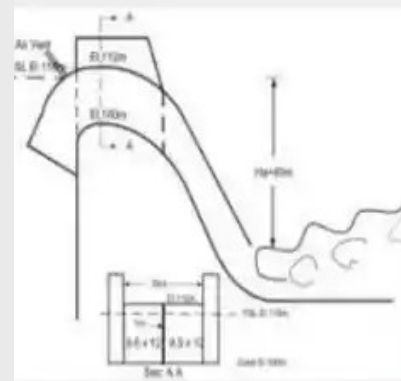


for different needs:

- *electricity production*
- *water supply*
- *irrigation*
- *other*

Basically, Spillways ensure a safe passage of floods from the reservoir into the downstream river reach.

Different designs of spillways



depend on:

- the design flood,
- dam type and location
- reservoir size and operation

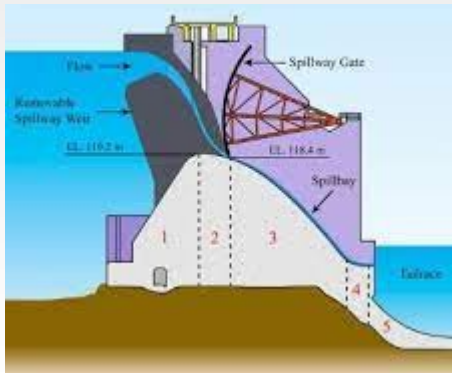


Classification of spillways

1. according to the function spillways are: **main service**, **emergency** and **auxiliary** ;



2. according to mode of control: **free (uncontrolled)** or **gated (controlled)** spillways



3. according to hydraulic criteria, i.e. type:

Overfall



Side channel



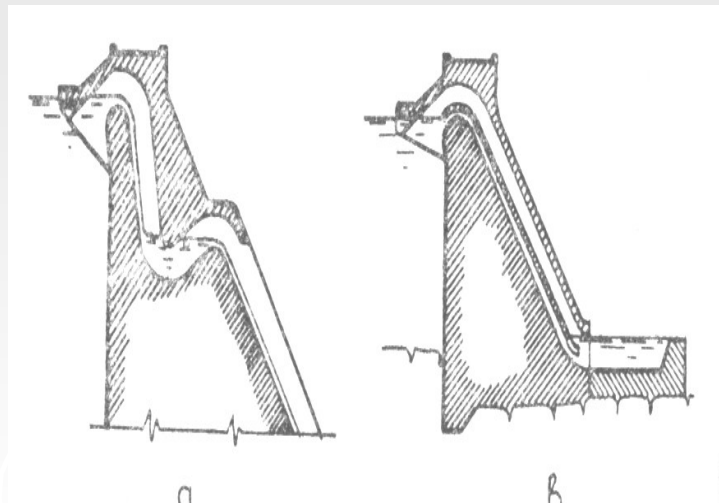
Chute



Shaft



Siphon



Tunnel



Selection of a spillway

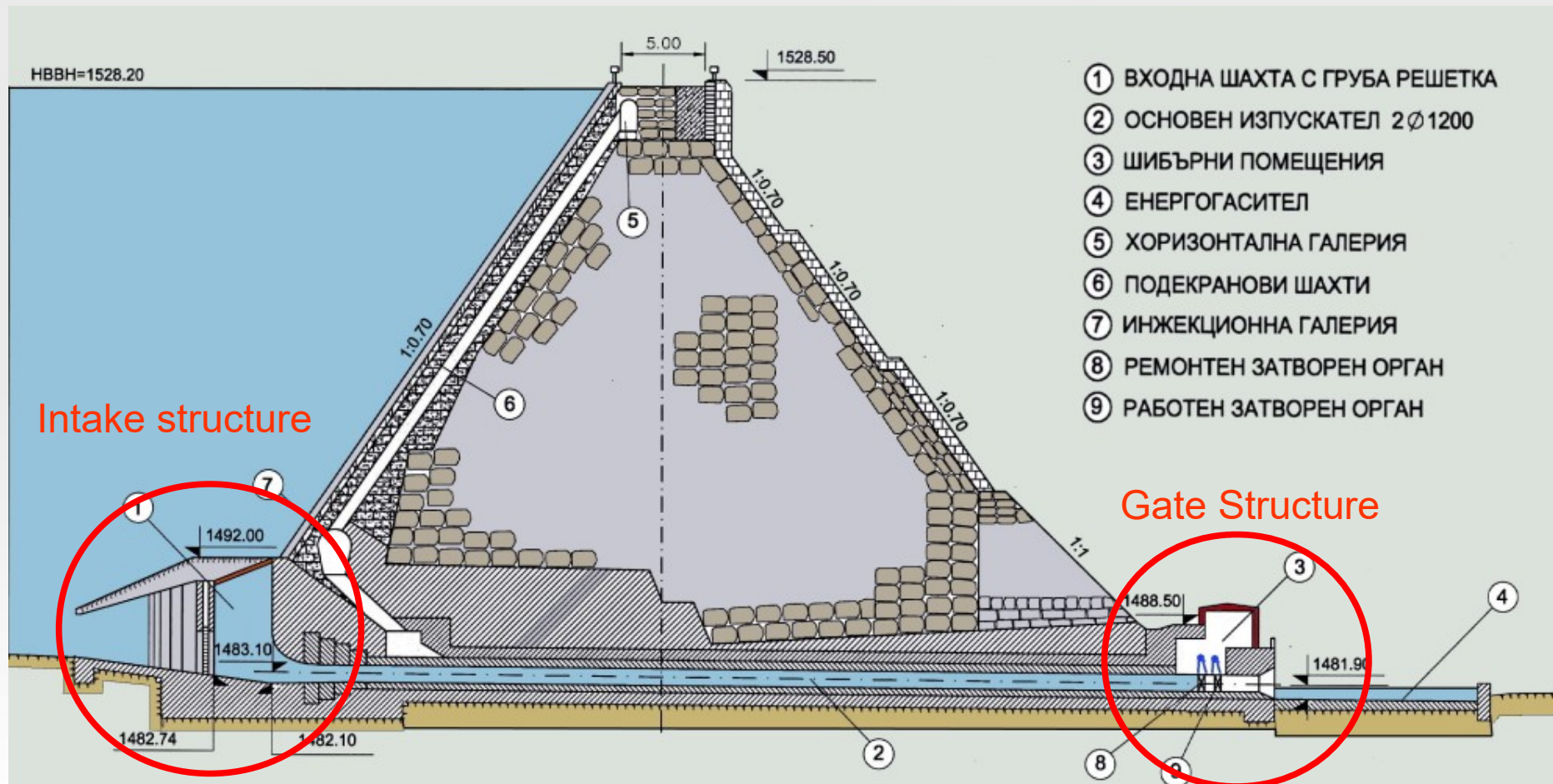
Main factors governing the choice of spillway are:

- 1) the reliability and accuracy of flood prediction,
- 2) the duration and amount of spillage,
- 3) seismicity of project site, topography and geology,
- 4) the dam type.



The design of bottom outlet depends on:

- the topography;
- the sediment inflow and deposition in the reservoir.



Day 09: #1

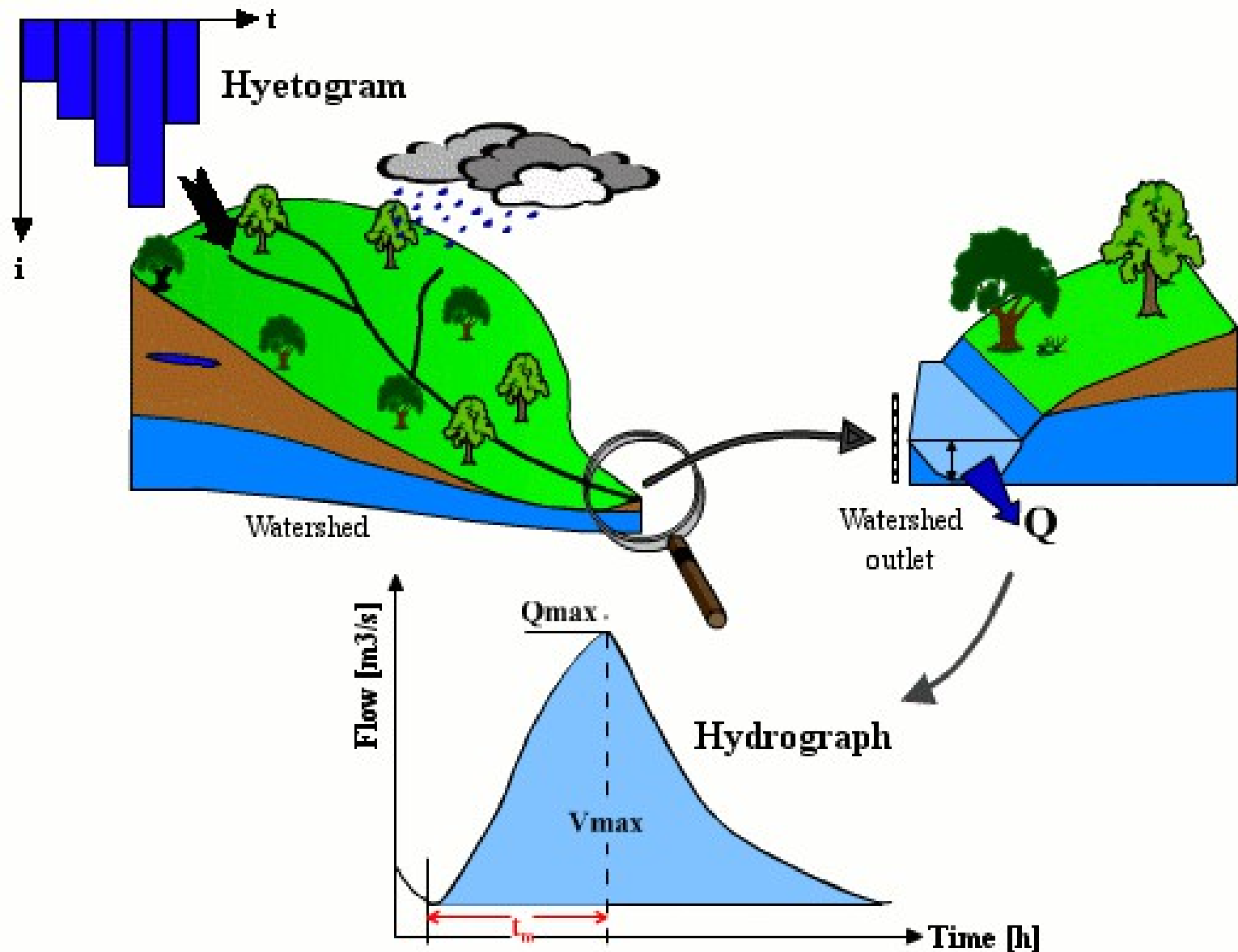
Watching video!

What is a spillway and how does it work?

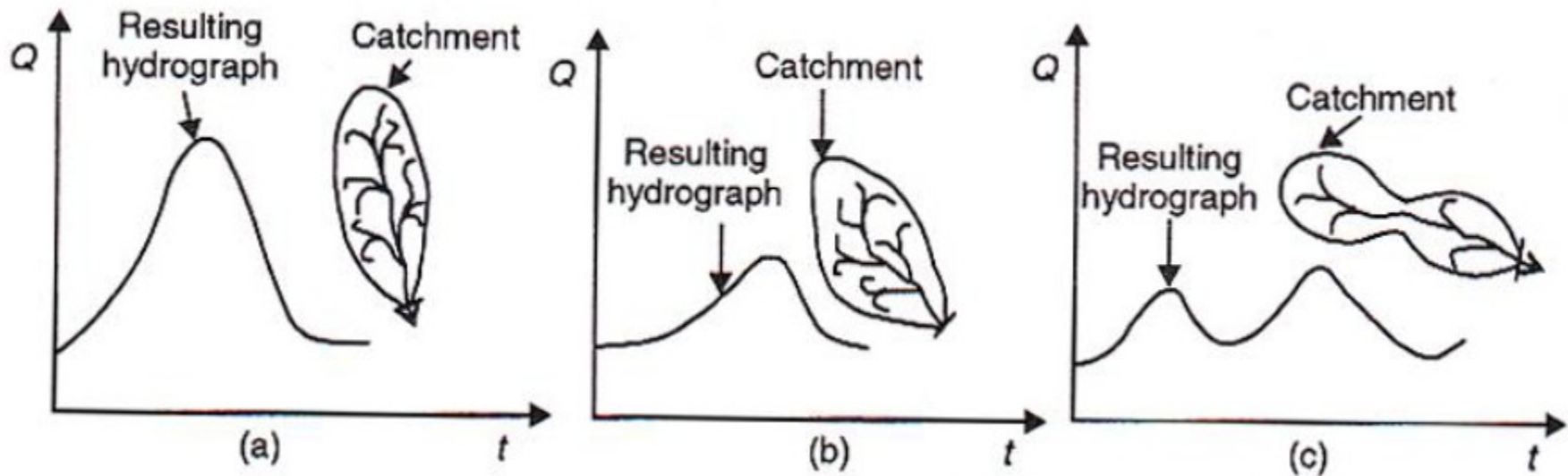
<https://www.youtube.com/watch?v=gLjabuiMqXE>



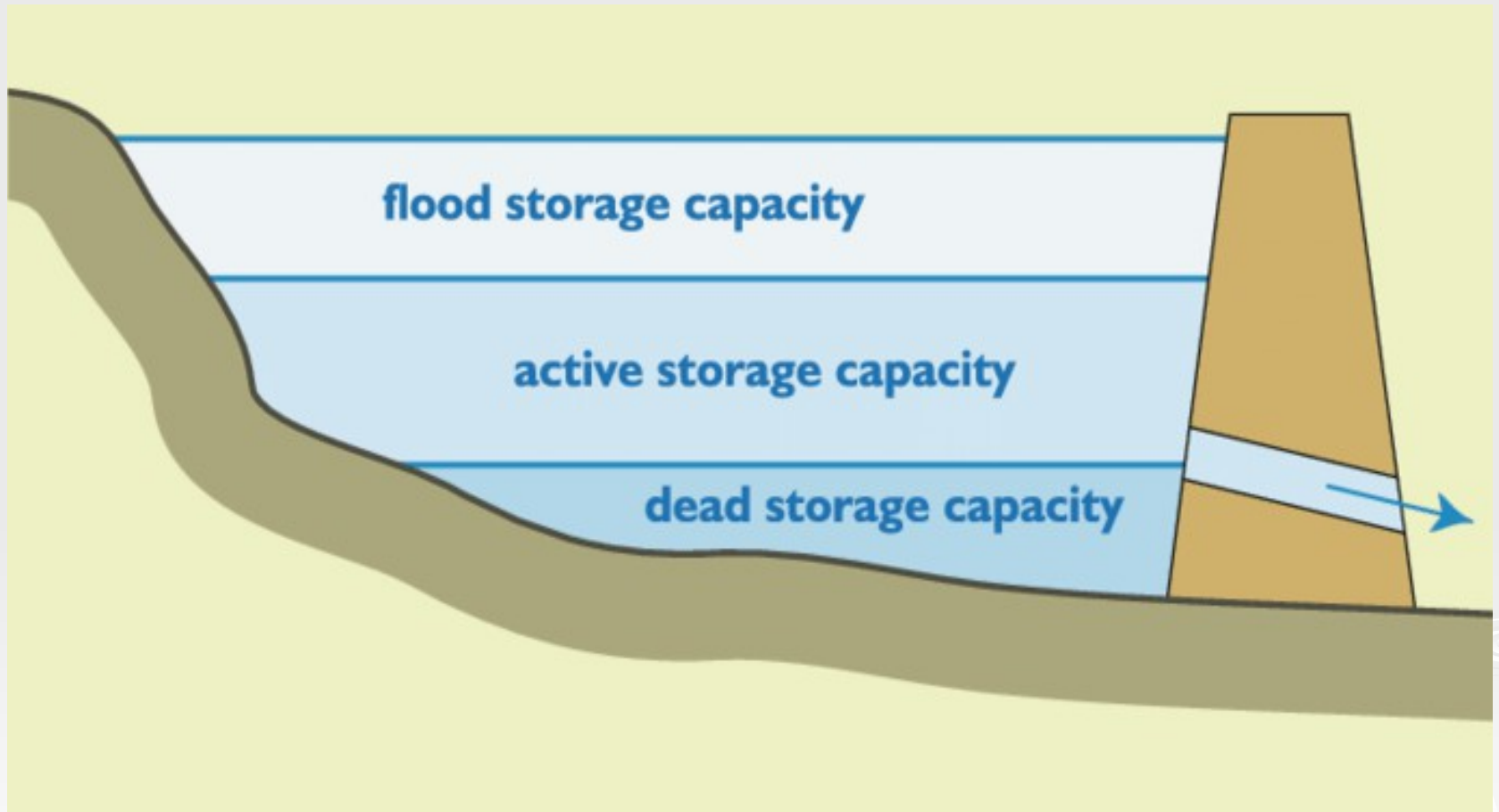
Design flood



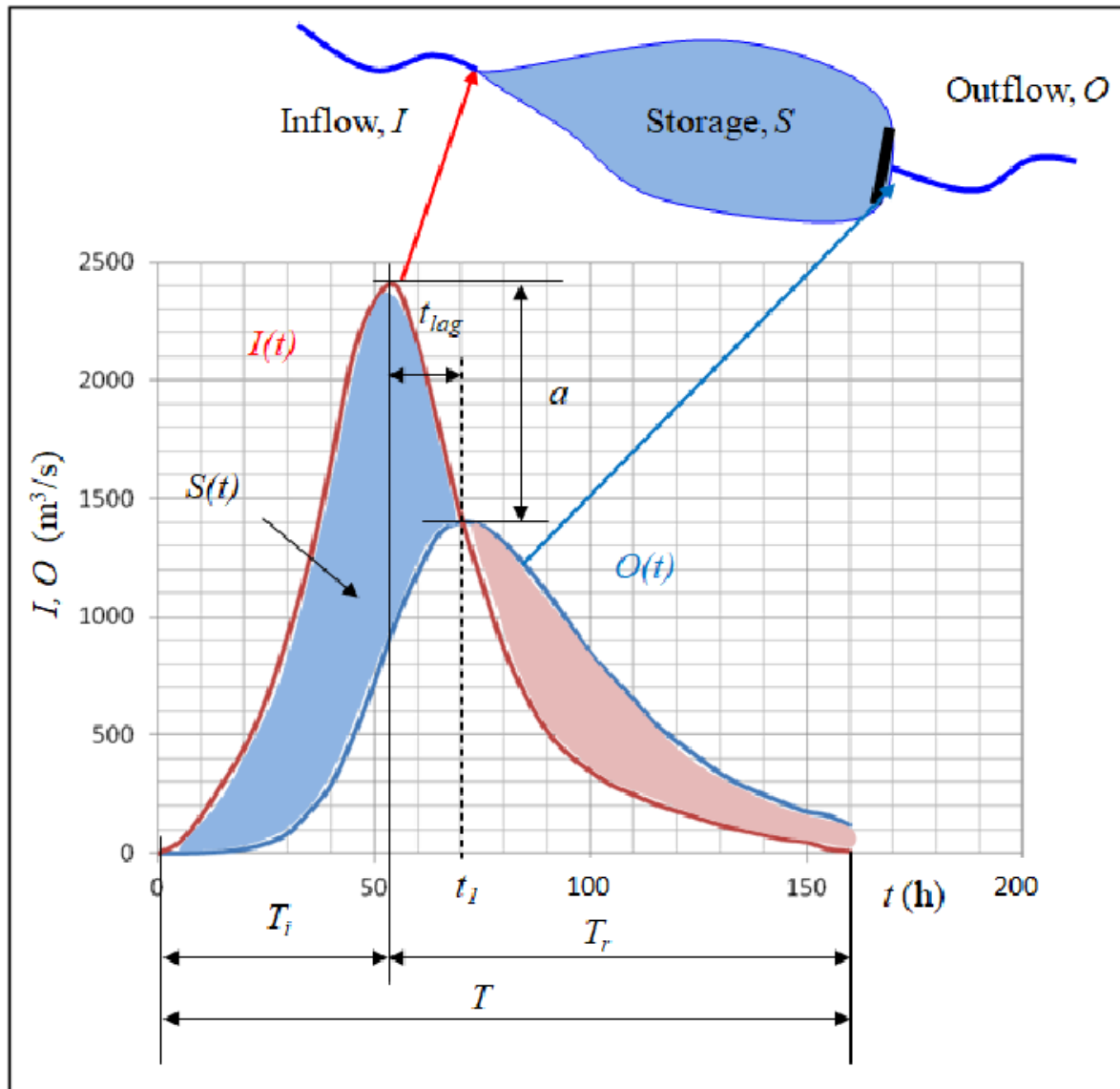
Class of the Dam	Design Flood Probability
1 st class	P=0,01 % (1 in 10 000 years)
2 nd class	P=0,1 % (1 in 1000 years)
3 th class	P=0,5 % (1 in 200 years)
4 th class	P=1 % (1 in 100 years)



Flood routing by the storage capacity of a reservoir



Schematic of a reservoir routing process



Balance equation

$$V_{in} = \Delta V - V_{spill} - V_{bo} + V_p - V_f - V_e,$$

where:

V_{in} – volume of the inflow hydrograph ;

ΔV – change in the volume of the reservoir ;

V_{spill} – outflow volume trough the spillway ;

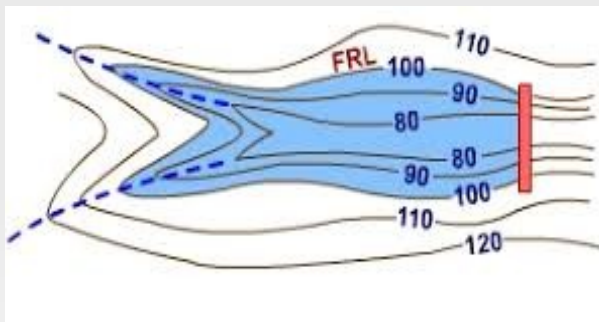
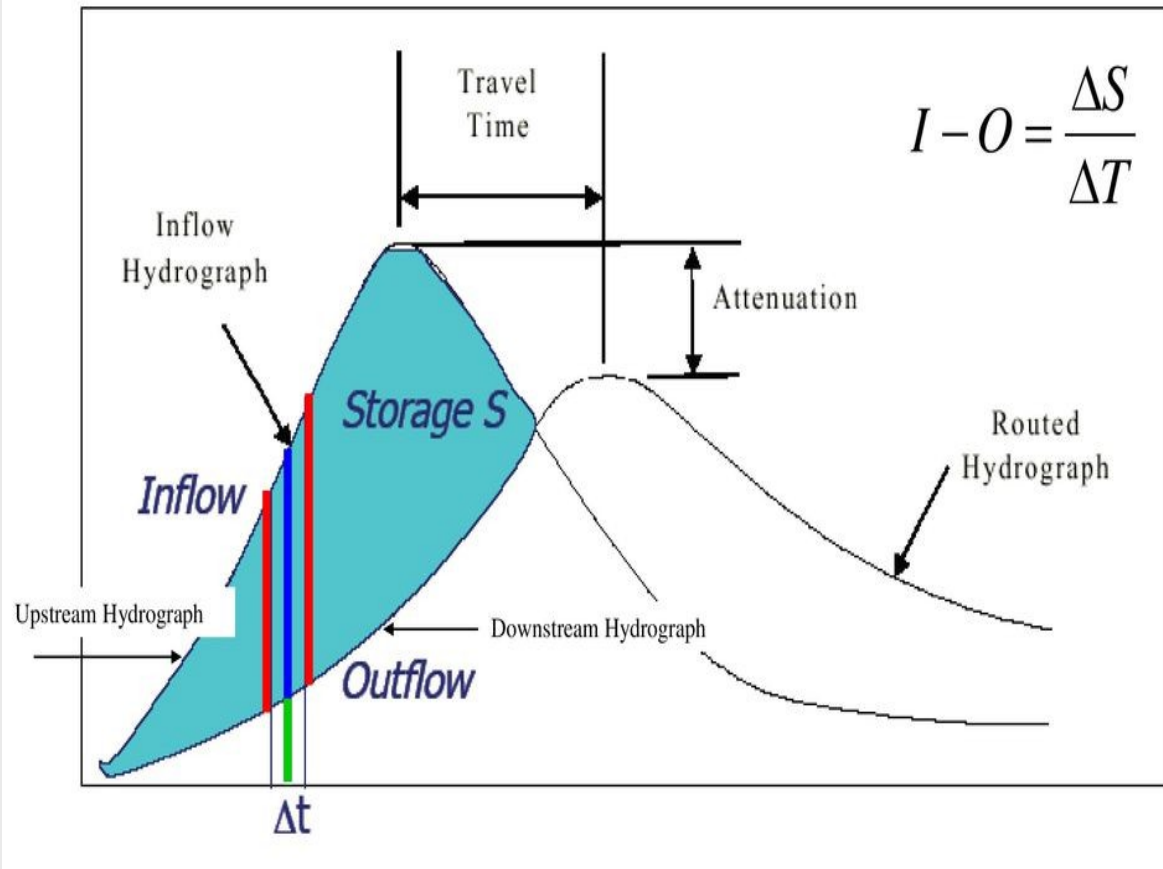
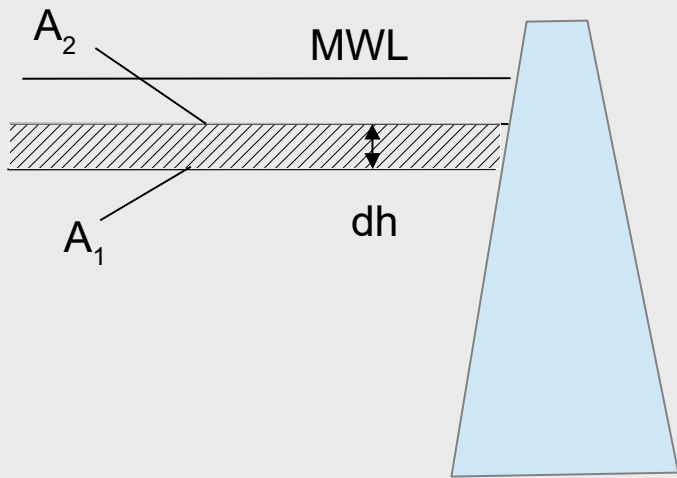
V_{bo} – outflow volume trough the other outlet works; (=0)

V_p – volume of dirrect rainfall on the storage area ; (=0)

V_f – outflow volume by infiltration; (=0)

V_e – outflow volume by evapotranspiration (=0)

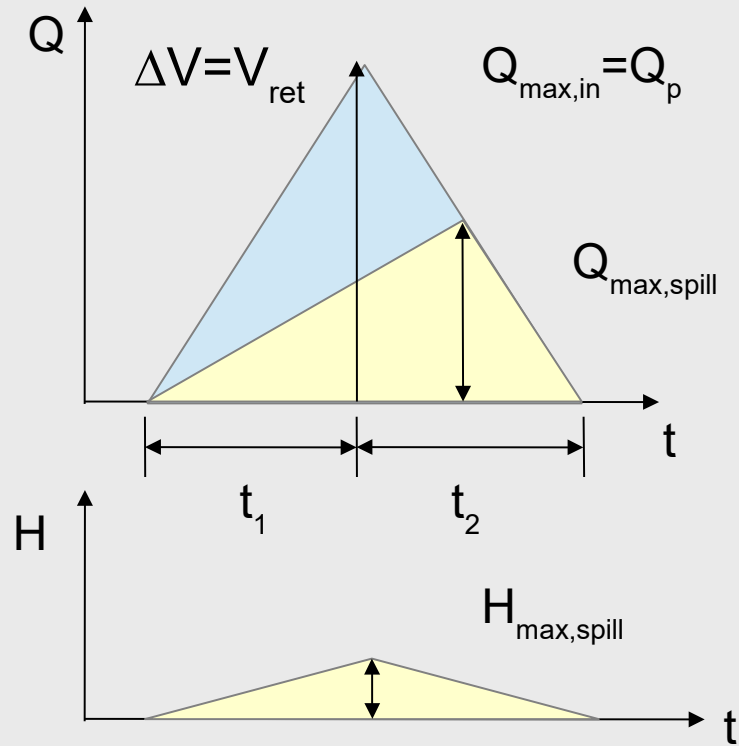
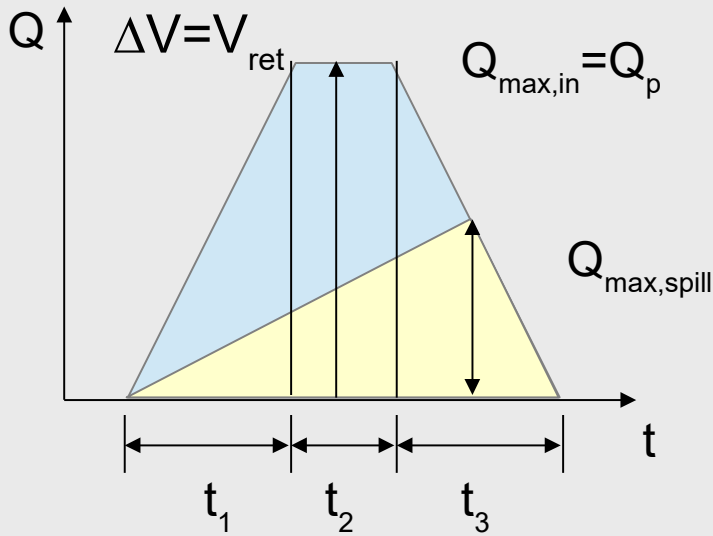
$$V_{in} = \Delta V - V_{spill}$$



Contours of storage areas - A

$$dQ_{inflow} dt = A dh + dQ_{outflow} dt$$

$$A = (A_1 + A_2) / 2$$



$\Delta V = V_{ret} = V_{in} - V_{spill}$ - area of blue trapezoid or triangle

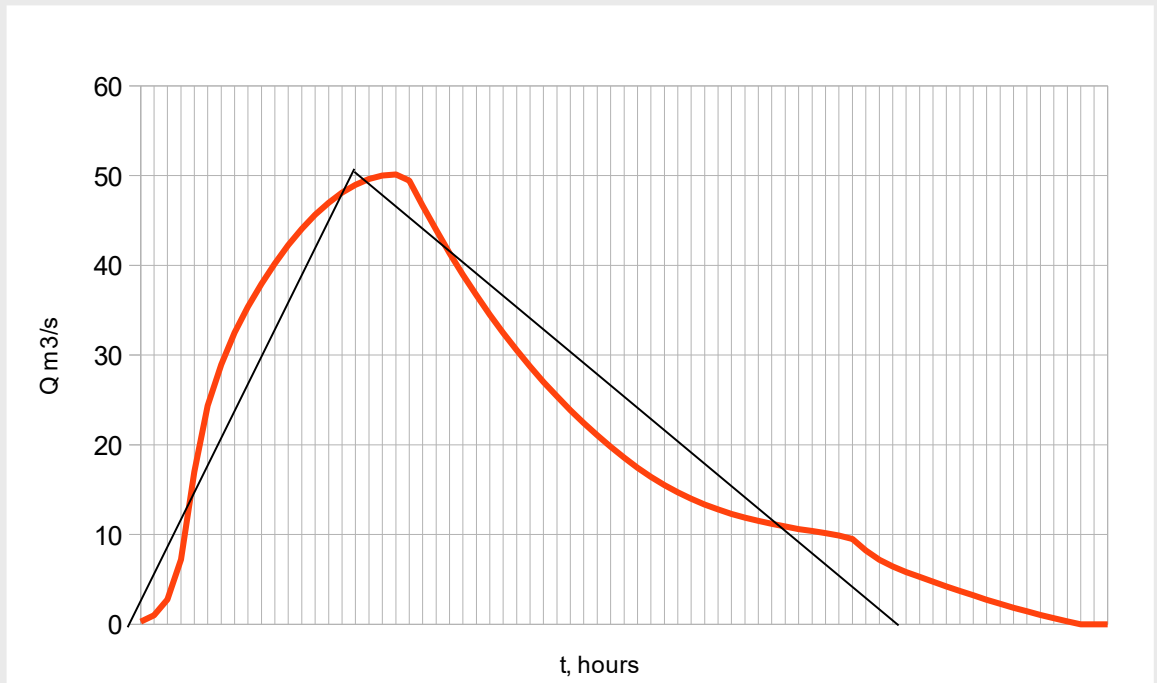
$$V_{in} = 0,5 Q_p (t_1 + t_2)$$

$$V_{spill} = 0,5 Q_{max, spill} (t_1 + t_2) - \text{area of yellow triangle}$$

$$V_{ret} = V_{in} - Q_{max, spill} (V_{in} / Q_p)$$

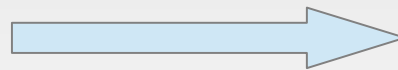
$$V_{ret} = A \cdot H_{max}$$

$$Q_p = 50,12 \text{ m}^3/\text{s}$$
$$V_{in} = 5279906 \text{ m}^3$$

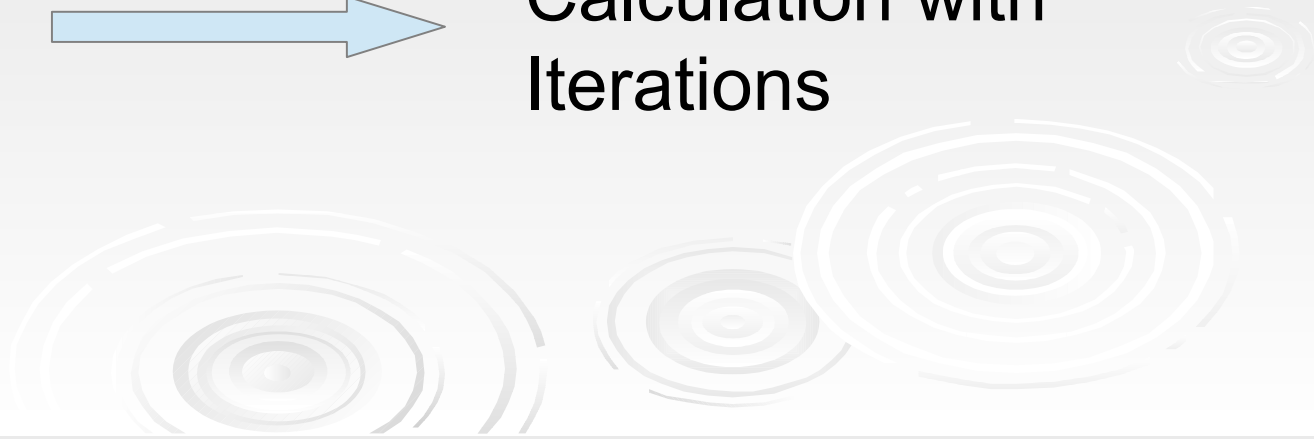


$$V_{spill} = 0,5Q_{max, spill}(t_1 + t_2)$$

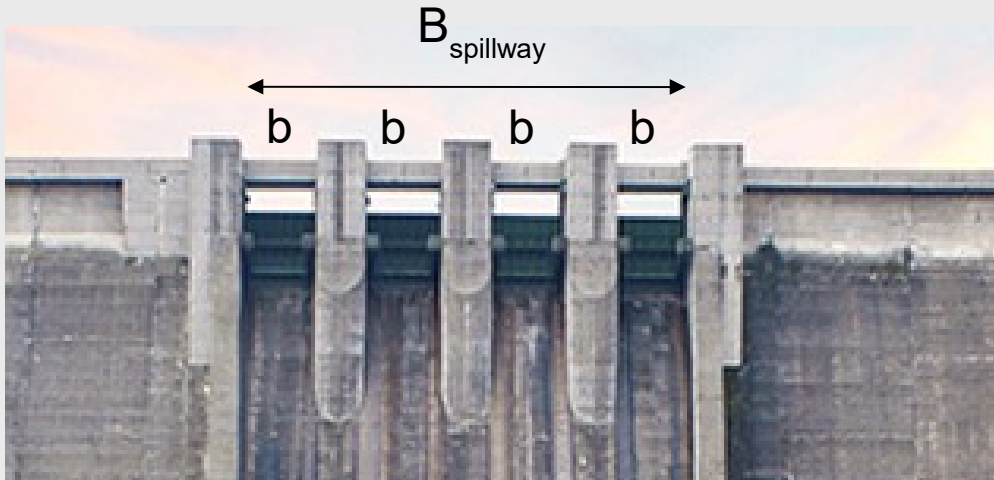
$$Q_{max, spill} = ?$$



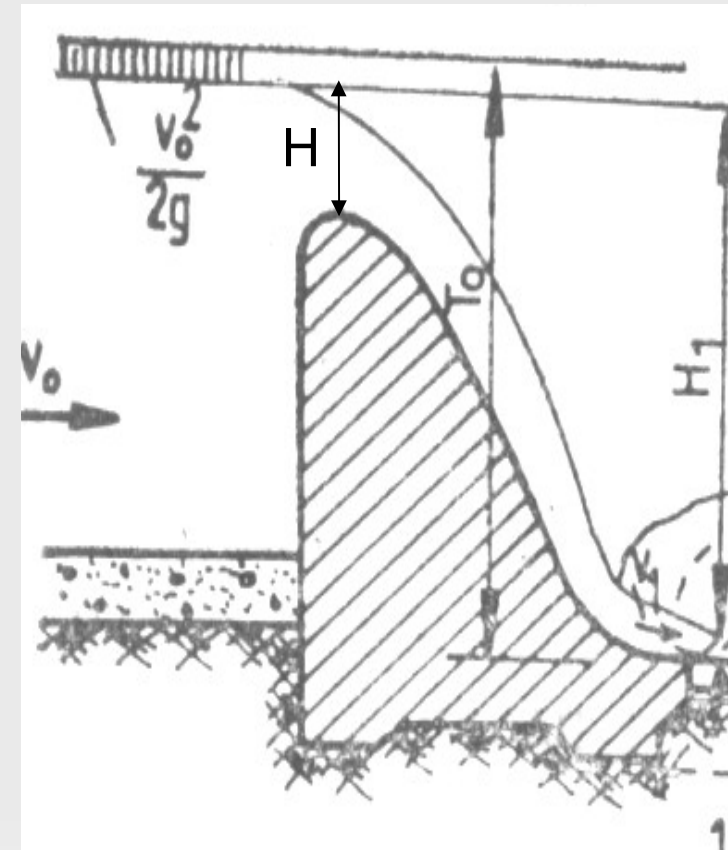
Calculation with
Iterations



Spillway Design

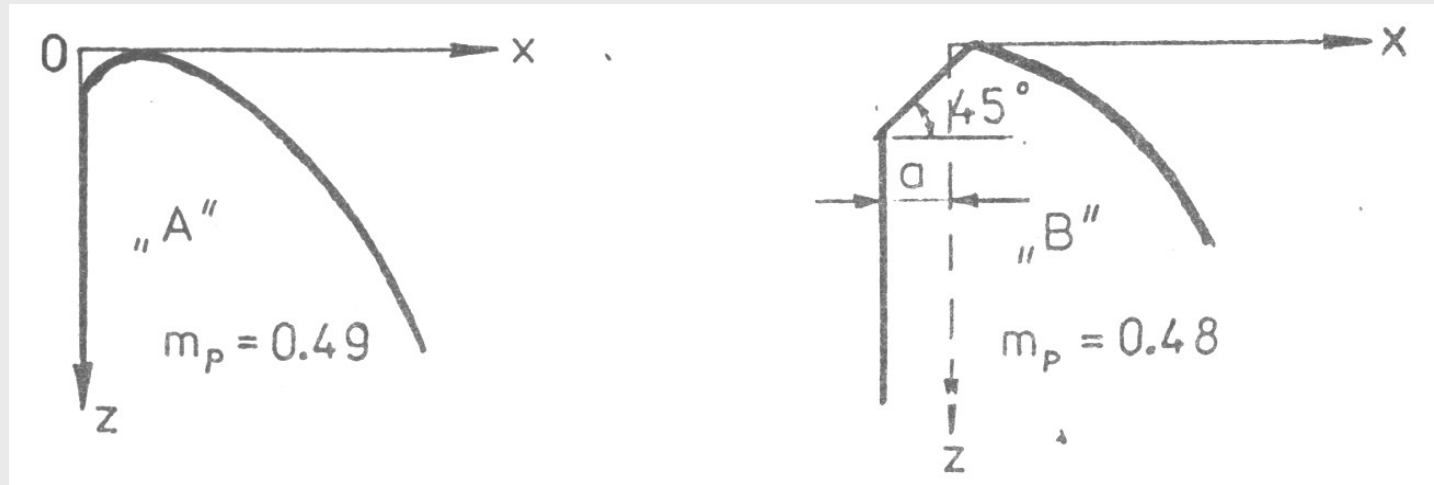


Overfall spillway (Ogee spillway)



$$Q = m \sum b \sqrt{2g} H^{\frac{3}{2}}, \text{ where: } H = H + \frac{\alpha v^2}{2g}$$

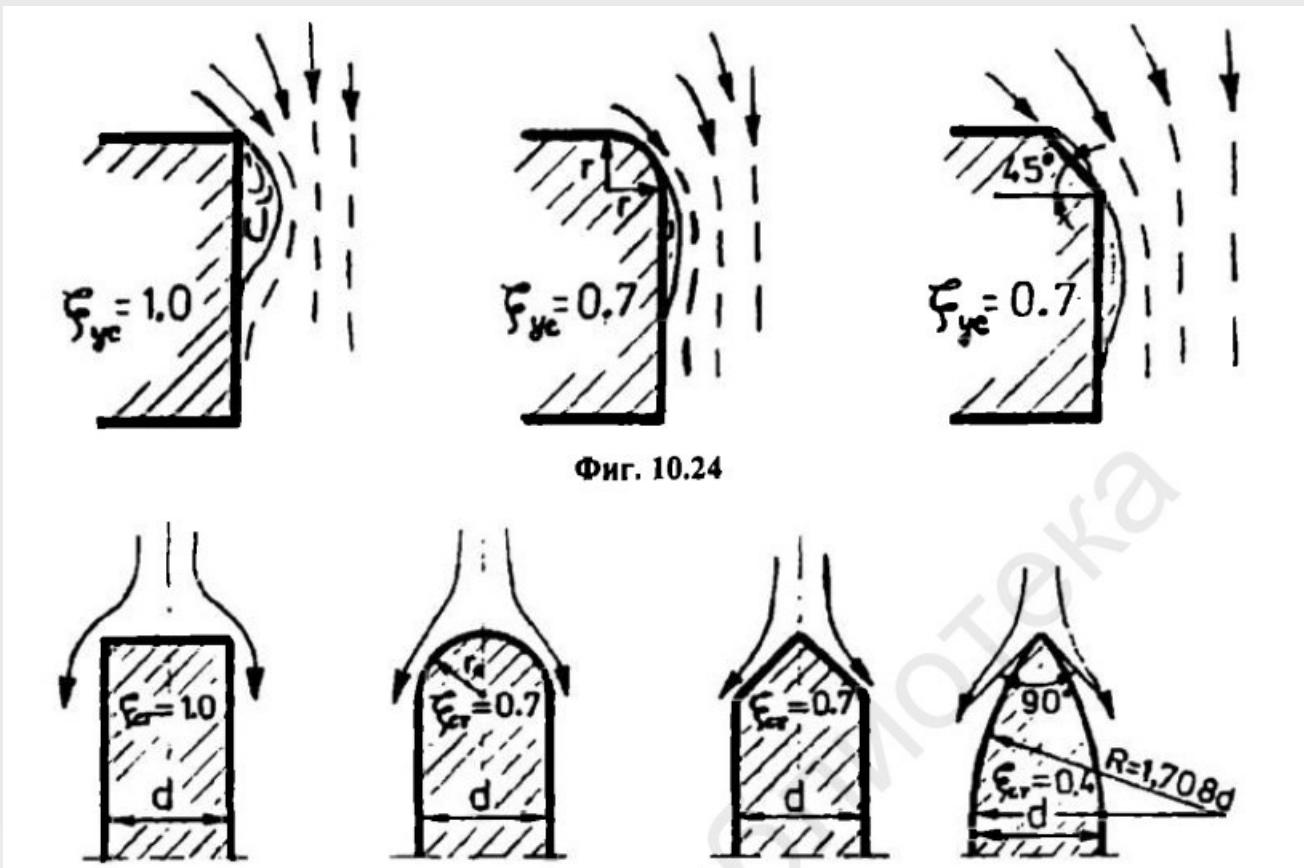
Standard spillway crest by Kriger-Officerov case A and B:



$m_p = m$ is coefficient of discharge

the spillway coordinates $/x, z/$ have to multiply by H ;

x	z		x	z		x	z	
	тип А	тип В		тип А	тип В		тип А	тип В
1	2	3	4	5	6	7	8	9
0.0	0.126	0.043	0.8	0.146	0.189	2.5	1.960	2.14
0.1	0.036	0.010	1.0	0.256	0.321	3.0	2.824	3.06
0.2	0.007	0.000	1.2	0.394	0.480	3.5	3.818	4.08
0.3	0.000	0.005	1.4	0.564	0.665	4.0	4.938	5.24
0.4	0.006	0.023	1.7	0.873	0.992	4.5	6.220	6.58
0.6	0.060	0.090	2.0	1.235	1.377			



Фиг. 10.24

$$Q = m \epsilon b \sqrt{2g} H_0^{3/2}$$

$$\epsilon = 1 - 0.2 \frac{\xi_{yc} + (n-1)\xi_{ст}}{nb} H_0$$

n – number of opens

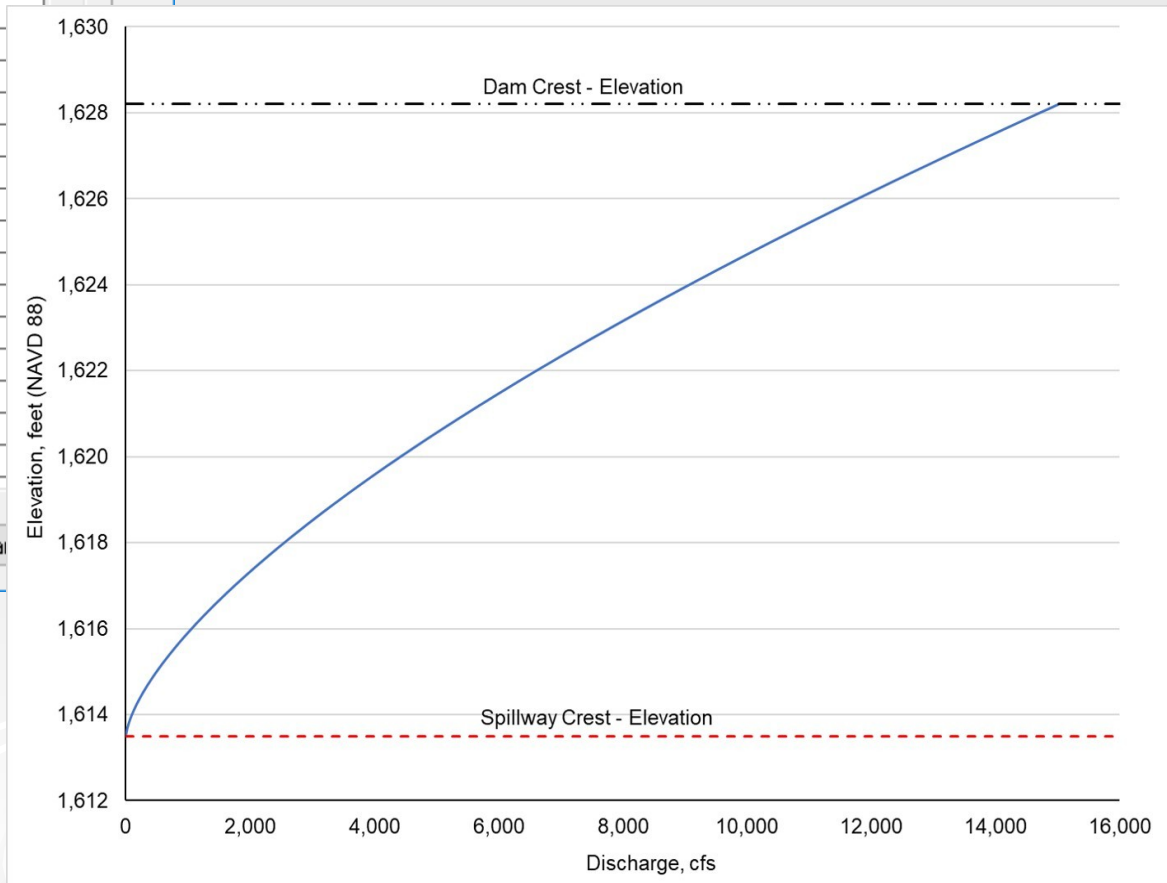
Drawing rating curve of a spillway

Spillway Level-Discharge: Node A1

	US level m	Discharge m ³ /s
1		
2		
3		
4		
5		
6		
7		
8		
9		
10		
11		
12		
13		
14		
15		

OK Graph Cal

$$Q = m \sum b \sqrt{2gH}^{\frac{3}{2}}$$



How to elaborate a rating curve of a spillway?

Input data:

$b = \dots$ m – spillway wide

$n = \dots$ – number of opening

$H = \dots$ m – spillway water level

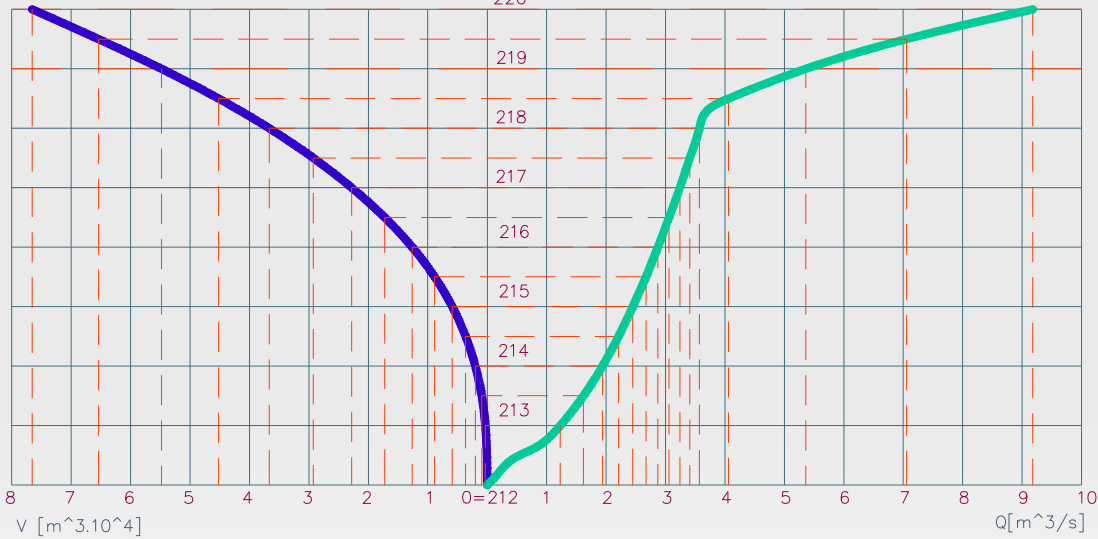
Please solve the spillway equation in a table as you start with $H=0$ to $H=H_{\max}$! After that draw a rating curve!

$$Q = m \sum b \sqrt{2gH^{\frac{3}{2}}}, \text{ where } : H = H + \frac{\alpha v^2}{2g} \quad 0$$

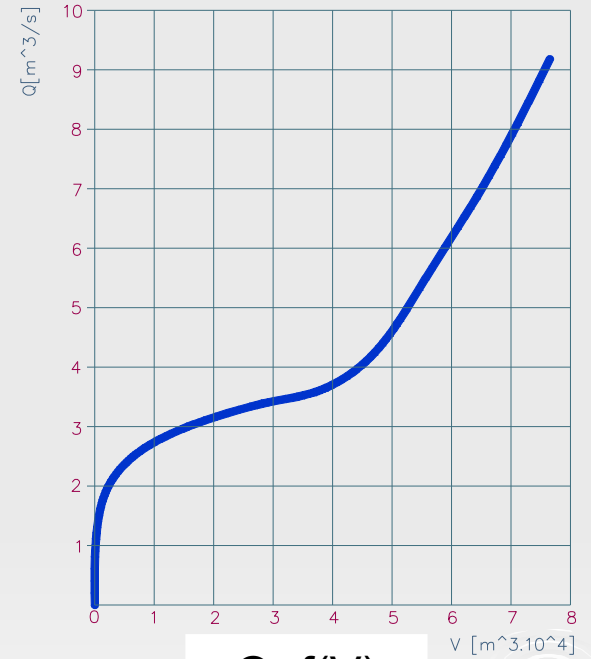
Please solve the balance equation and define the V_{ret} !

Graphical method for flood routing

Storage curve

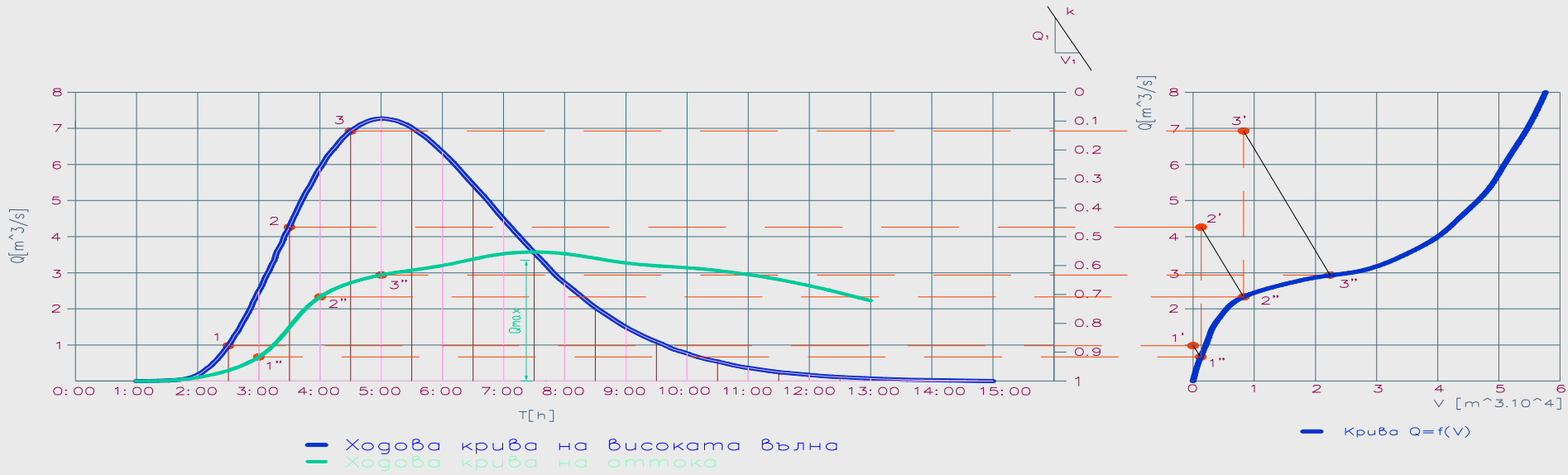


Rating curve



$Q=f(V)$

Graphical method for flood routing

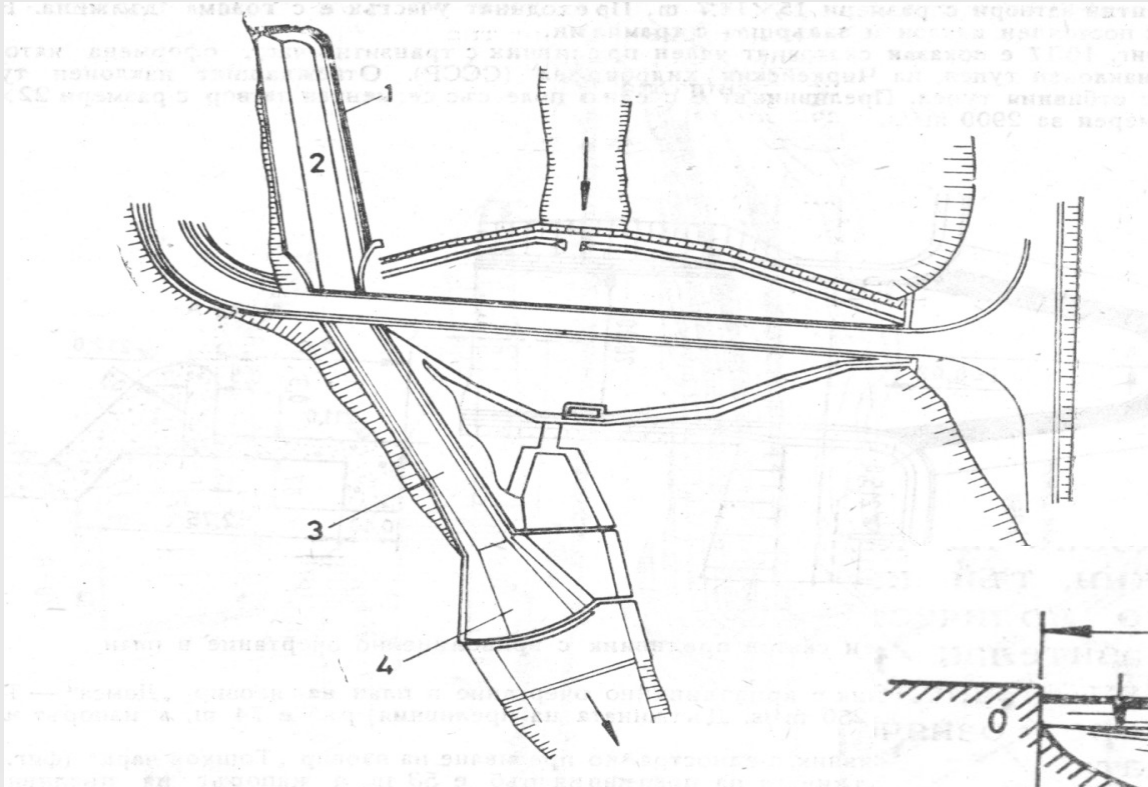


Day 09: #3

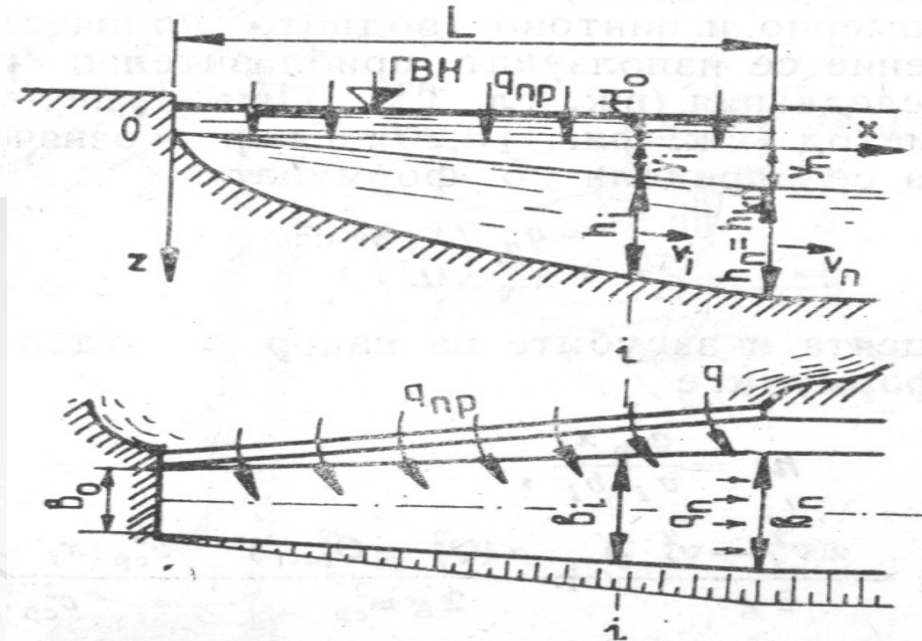
Exercise:

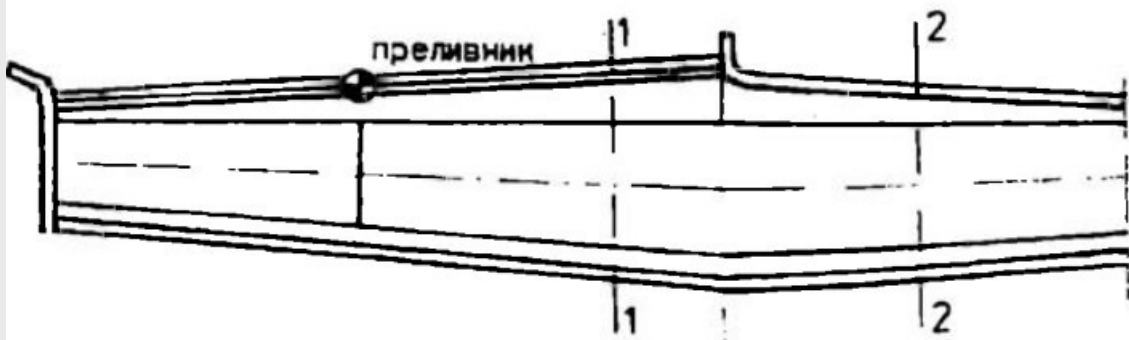
Drawing routed hydrograph

Side spillway



- 1 – spillway crest
- 2 – trench
- 3 – chute
- 4 – stilling basin

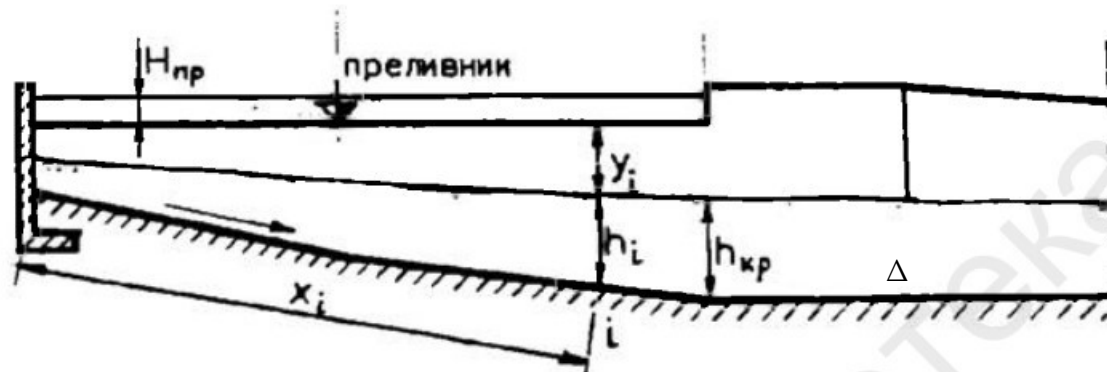




$$v_i = a x_i^n; \quad a = \frac{v_{кр}}{L_{пр}^n};$$

$$y_i = \frac{n+1}{n} \frac{v_i^2}{2g};$$

$$\Delta y_i = \frac{Q_{i-1}}{g} \frac{v_{i-1} + v_i}{Q_{i-1} + Q_i} \left(\Delta v_i + \frac{q_* v_i \Delta x}{Q_{i-1}} \right)$$



V_i – velocity in i -section;
 $V_{кр}$ – critical velocity at the end of the trench

ΔV_i – change in velocity
 between each two sections

Δx – distance between two
 sections

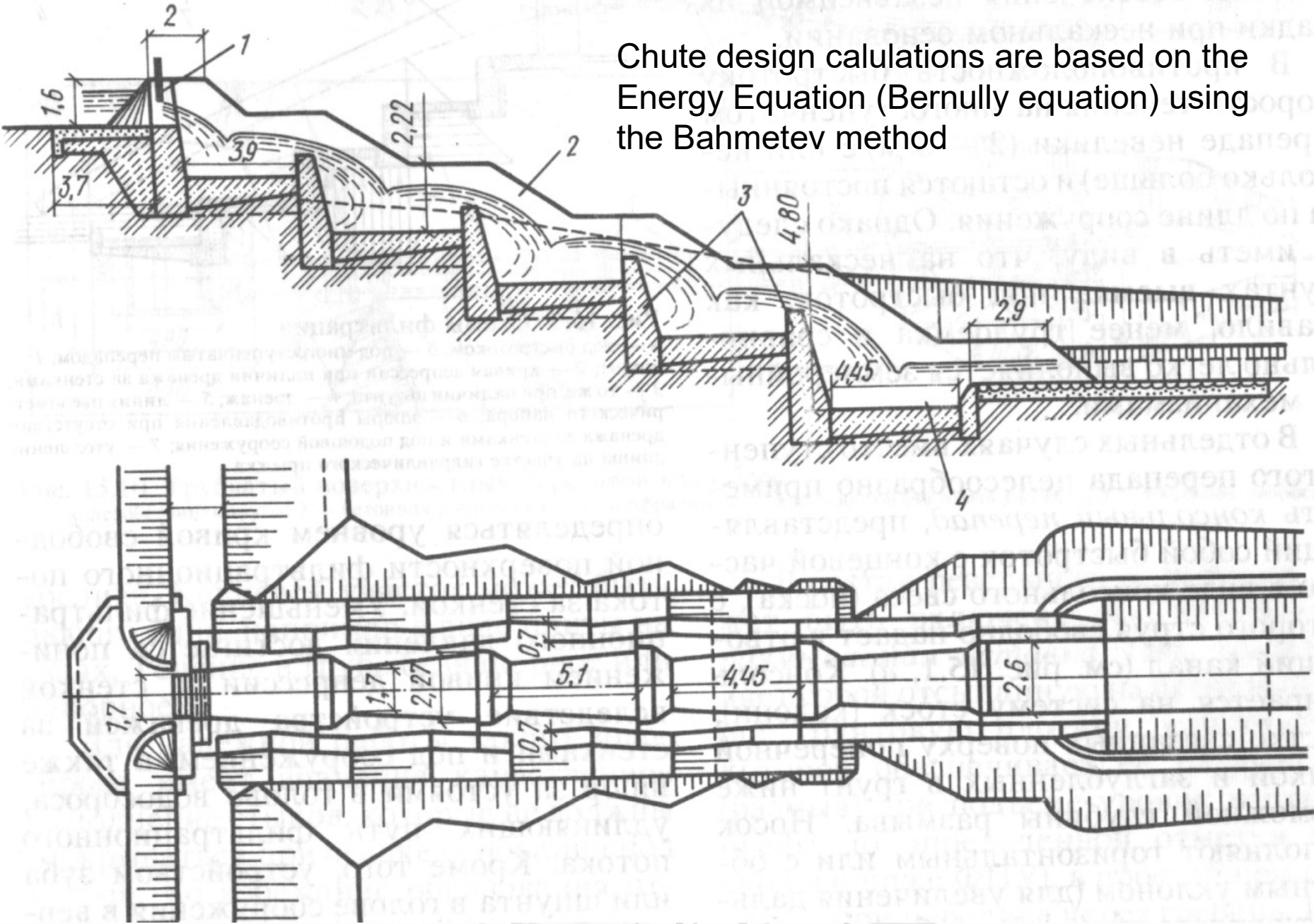
Δy_i – reduction of the water
 level between two sections



The trench is separated in 5-10 sections and the parameters below are calculated between each two sections.

$$Q_i = q_* x_i; \quad \omega_i = \frac{Q}{v_i}; \quad h_i = \frac{\omega_i}{b_i}; \quad y_i \quad \text{и} \quad H_i = h_i + y_i$$

Chute design calculations are based on the Energy Equation (Bernully equation) using the Bahmetev method



Frictionless Flow: The Bernoulli Equation

- Closely to the steady flow energy equation is a relation between pressure, velocity, and elevation in a frictionless flow, now called the *Bernoulli Equation*.
- For an unsteady frictionless flow

$$\int_1^2 \frac{\partial V}{\partial t} ds + \int_1^2 \frac{dp}{\rho} + \frac{1}{2} (V_2^2 - V_1^2) + g(z_2 - z_1) = 0$$

- For steady frictionless flow

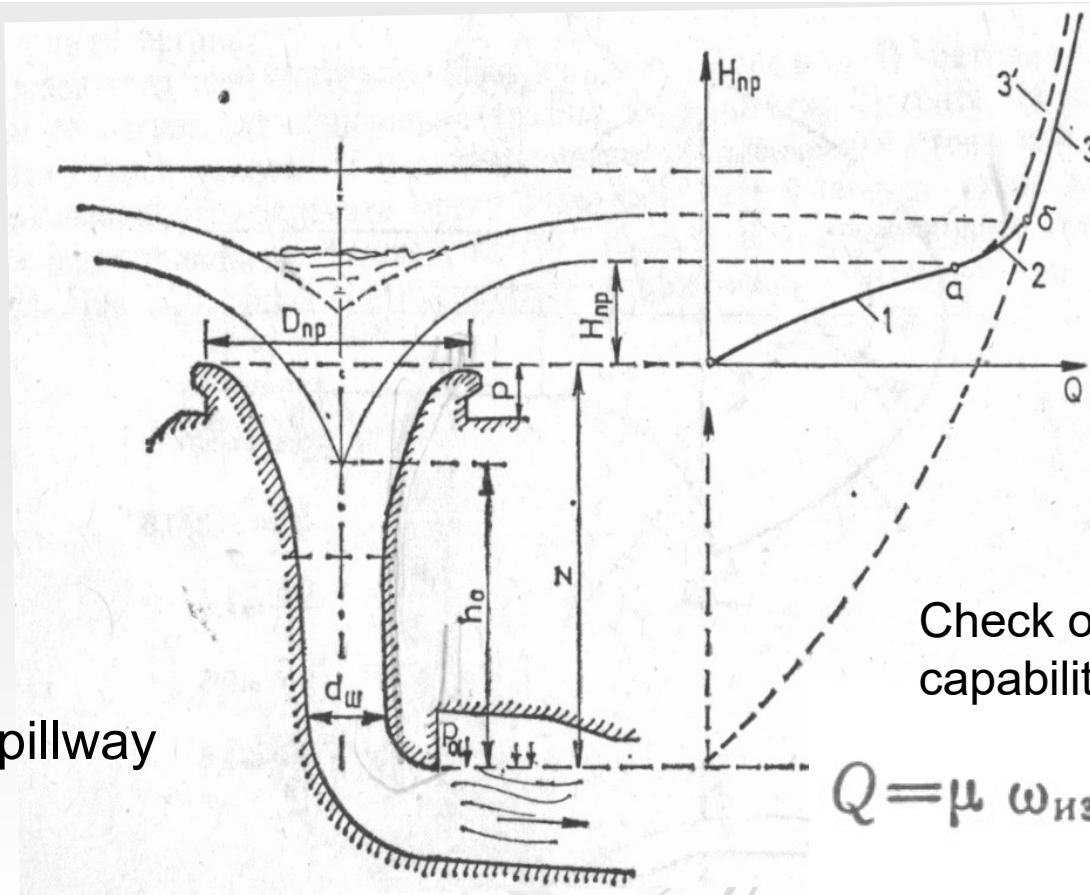
$$\frac{p_2 - p_1}{\rho} + \frac{1}{2} (V_2^2 - V_1^2) + g(z_2 - z_1) = 0$$
$$\frac{p_1}{\rho} + \frac{1}{2} V_1^2 + gz_1 = \frac{p_2}{\rho} + \frac{1}{2} V_2^2 + gz_2 = \text{const}$$

- in case of k-numbers of pillars with width „d“

$$1) \quad Q = m_{06} \varepsilon (2 \pi R - k d) \sqrt{2 g} H_{np}^{3|2};$$

- in case of no pillars

$$2) \quad Q = m_{06} 2 \pi R \sqrt{2 g} H_{np}^{3|2}.$$



Shaft spillway

Check of the spillway flow capability

$$Q = \mu \omega_{нзх} \sqrt{2 g (H_{np} + z)}$$